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دانشجویان با اطلاعات علمی بین‌المللی و ترفند های جستجو
Prediction of Hydraulic Efficiency of Primary Rectangular Settling Tanks Using the Non-linear $k - \varepsilon$ Turbulence Model

B. Firoozabadi$^{1,*}$ and M.A. Ashjari$^1$

**Abstract.** Circulation is created in some parts of settling tanks. It can increase the mixing level, decrease the effective settling, and create a short-circuiting from the inlet to the outlet. All above-mentioned phenomena act in such a way to decrease the tank's hydraulic efficiency, which quantitatively shows how flow within the tank is uniform and quiet. So, the main objective of the tank design process is to avoid forming the circulation zone, which is known as the dead zone. Prediction of the flow field and size of the recirculation zone is the first step in the design of settling tanks. In the present paper, the non-linear $k - \varepsilon$ turbulence model is used for predicting the length of the reattachment point in the separated flow of a Karlsruhe tank. Then, the recirculation bubble size, which is out of the capability of standard turbulence models, is determined. Also, the effect of the separation zone size on the tank's hydraulic efficiency is investigated.

**Keywords:** Settling tanks; Non-linear $k - \varepsilon$; FTC calculation; Hydraulic efficiency; Circulation.

**INTRODUCTION**

Settling tanks have been used for separating floating particles from the main flow. To optimize the operation of these tanks, it is required that a quiet flow of fluid be formed in the tank in such a way that the sedimentation process be performed in the best way. However, creation of the dead zone and the short-circuiting of the inlet to the outlet disturb the quietness of flow and leave negative influences on the performance of the tank. To prevent these phenomena, some baffles are used in several parts of the tank. By optimizing the application of the baffles in the tanks, the size of the recirculation zones is reduced and the plug flow percentage, which is the ideal flow for clarifiers, is increased. In fact, several parameters, e.g., location and type of inflow and effluent, location and size of baffle, and rate of sludge withdrawal could influence the efficiency of settling tanks. Then, the best way to determine their order of magnitude is to develop a suitable mathematical model. Mathematical models have been mainly developed, since incorpora-

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1. School of Mechanical Engineering, Sharif University of Technology, Tehran, P.O. Box 11155-9563, Iran.
2. Corresponding author. E-mail: firoozabadi@sharif.edu

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shown good results in calculating the separation point length of backward-facing step flow. Nevertheless, overestimating the length of the recirculation zone in the tank was slightly surprising. In this flow, the formation of streamlines with strong curvature caused the modified model to work improperly.

Linear $k-\varepsilon$ turbulence models, which, in fact, can be thought of as tensorially invariant mixing length theories, usually work quite well in the description of unseparated turbulent boundary layers. Although in thin shear flows, normal Reynolds stresses do not play an important role, they have a main role in describing flows that involve recirculation zones. In these cases, incorporating the linear $k-\varepsilon$ models, which are completely incapable of accurately predicting normal Reynolds stresses, is not a very appropriate model.

Preliminary research taking into account the effects of non-linear terms and modifying the $k-\varepsilon$ model was undertaken by Lauder and Ying [6] and Rodi [7]. However, the obtained models were developed in such a way that general invariance was not exhibited and, hence, good results only showed for special flows. It is very interesting that the main steps in developing a general non-linear turbulence model had been taken many years ago by Rivlin [8] and Lumley [9]. They had focused on a clue whereby there exist striking similarities between the main turbulent flow of a Newtonian fluid and the laminar flow of viscoelastic fluids. Continuation of their method by further researchers caused the desired model, known as non-linear or anisotropic $k-\varepsilon$, to be obtained by Speziale [10]. A few years later, Lauder [11] offered a very complete form of the model that contained higher order terms. In addition, these non-linear models have all advantages of the linear $k-\varepsilon$ model; they can better describe the normal stresses and can be incorporated into most $k-\varepsilon$ model computer codes in a relatively simple manner.

In this paper, some numerical problems that occur through the appearance of non-linear terms in the anisotropic turbulence model will be introduced. In addition, the size of the recirculation bubble in the Karlsruhe basin will be determined by using the non-linear $k-\varepsilon$ model and comparing it with Adams and Rodi’s [4] experimental measurements. Finally, the performance of the non-linear model in predicting the hydraulic efficiency and FTC of the tank, which are strongly affected by the velocity field and recirculation size, will be investigated.

**MATHEMATICAL MODEL**

The steady state, two-dimensional flow in the Karlsruhe basin is determined using time averaged conservation equations. The Navier-Stokes and continuity equations for Newtonian incompressible fluid are expressed as:

$$\frac{\partial u}{\partial x} + \frac{\partial v}{\partial y} = 0,$$

$$\frac{\partial u}{\partial x} + \frac{\partial u}{\partial y} = -\frac{\partial p}{\partial x} + \nu \left( \frac{\partial^2 u}{\partial x^2} + \frac{\partial^2 u}{\partial y^2} \right),$$

$$\frac{\partial v}{\partial x} + \frac{\partial v}{\partial y} = -\frac{\partial p}{\partial y} + \nu \left( \frac{\partial^2 v}{\partial x^2} + \frac{\partial^2 v}{\partial y^2} \right),$$

where $u$ and $v$ are the mean velocity components in the $x$ and $y$ directions; $p$ is the modified mean pressure defined by Speziale and Thauang [12]; $\tau_{11}$, $\tau_{12}$ and $\tau_{22}$ are normal and shear components of the Reynolds stresses, respectively, and $\nu$ is the kinematic viscosity of the fluid (here, it is water at $20^\circ$C). The Reynolds stresses could be modeled via an eddy viscosity assumption based on Speziale [10] in the form:

$$\tau_{ij} = \frac{2}{3} k \delta_{ij} - 2 C_\mu \frac{k^2}{\varepsilon} S_{ij} - 4 C_D C_\mu \frac{k^3}{\varepsilon^2} \times \left( S_{ij}^p - \frac{1}{3} S_{kk} S_{ij} + S_{ik} S_{kj} - \frac{1}{3} S_{kl} S_{lk} \delta_{ij} \right),$$

where subscripts $i, j = 1$ and $2$ stand for $x$ and $y$, respectively; $k$ is the kinetic energy of turbulence; $\varepsilon$ is the turbulent dissipation rate, and $C_\mu$ is a dimensionless constant that is taken to be 0.09, as offered by Rodi [1]. $C_D$ is the dimensionless constant, defined as 1.63, based on calibration of the non-linear model with Lauder’s [13] experimental data for fully developed turbulent channel flow; $\delta_{ij}$ is the Kronecker delta, and $S_{ij}$, the mean rate of strain tensor, is expressed by:

$$S_{ij} = \frac{1}{2} \left( \frac{\partial u_i}{\partial x_j} + \frac{\partial u_j}{\partial x_i} \right),$$

$S_{ij}^p$ is the frame-indifferent Oldroyd derivative determined by :

$$S_{ij}^p = u_k \frac{\partial S_{ij}}{\partial x_k} - \frac{\partial u_i}{\partial x_k} S_{kj} - \frac{\partial u_j}{\partial x_k} S_{ki}.$$
turbulence models, for \( k \) and \( \varepsilon \). It has been observed, however, that the Boussinesq hypothesis fails in a number of applications, such as separated turbulent flows. Bradshaw [16] has stated that this failure is due to the form of the stress strain relation rather than the inapplicability of the eddy viscosity approach. In particular, the third term in Equation 4 corrects the fundamental weaknesses of the Boussinesq relationship. As shown by Pope [17], the new relationship has a strong ability to capture normal stress anisotropy, high sensitivity to secondary strains, and an ability to generate excessive turbulence at separation zones. Detailed tests by Speziale [10], Suga [18], and Craft et al. [19] have shown that the effect of the last term in Equation 4 is a significant improvement in the prediction of the reattachment length of the separation region behind the backward facing step, with results similar in accuracy to those obtained using a Reynolds stress model. However, the main advantage of the non-linear model over more sophisticated turbulence models is that the time consuming solution of the stress equations, in the form of partial differential or algebraic, is not needed. In addition, the inter-relation between strain and stress has been retained within the differential equation, which increases the numerical stability of the model. Our numerical experiences have shown that the non-linear model of Speziale [10] has a strong capability of predicting separated flow within settling tanks, while its numerical implementation and stability is, approximately, as simple and as good as the standard model. Nevertheless, some finer grids and, consequently, more CPU time are needed to deal with non-linear terms.

It is worth mentioning that \( k \) and \( \varepsilon \), in the non-linear model, are calculated using the same transport equations incorporated in the standard \( k-\varepsilon \) model [1].

Through Equations 1 to 6, the flow field in the basin could be solved and, hence, the tank’s model is fully defined hydrodynamically. To complete the tank’s modeling, the determined velocity field must be introduced into the transport equation of the dye such that the concentration can be calculated. By supposing that the dye has no influence on the flow field, and is dispersed only via diffusion and convection mechanisms, the transport equation will be:

\[
\frac{\partial C}{\partial t} + \frac{\partial u C}{\partial x} + \frac{\partial v C}{\partial y} = \frac{\partial}{\partial x} \left( \Gamma_x \frac{\partial C}{\partial x} \right) + \frac{\partial}{\partial y} \left( \Gamma_y \frac{\partial C}{\partial y} \right),
\]

where \( C \) is the mean dye concentration and \( \Gamma_x \), the isotropic eddy diffusivity coefficient, is proportional to the eddy viscosity, \( \Gamma_x = \frac{\nu_x}{\sigma_x} \), according to the Reynolds analogy theory for mass and momentum transport. Here, proportional coefficient \( \sigma_x \) is the Schmidt number and is 0.7, as proposed by Launder [11], for free turbulent flows.

INITIAL AND BOUNDARY CONDITIONS

The hydrodynamics part of the problem is steady state and the initial condition for the velocity field is not very important. Nevertheless, for calculating the tank’s FTC, no dye must exist, initially, all over the tank (\( C = 0 \)).

By knowing the velocity at the first node above the solid boundary, the shear velocity is calculated via the law of the wall, in the standard two-layer form developed by Launder and Spalding [20], and the wall is taken as being smooth. Formally, the wall function must not be applied to separated-turbulent boundary layers, and many researchers like Avva et al. [21] tried to modify it in several ways. However, since the flow field is solved iteratively, and the separated regions occupy a small space, confined only at the tank corner, major errors do not appear to the overall flow field. After calculating the shear velocity, the values of \( k \) and \( \varepsilon \) at the adjacent node above the wall are determined by assuming local equilibrium for turbulent production and dissipation [1]. All boundary conditions at the inlet, outlet, and free surface of the model tank are applied as recommended by Celik et al. [3].

NUMERICAL SOLUTION PROCEDURE

A new computer code, in FORTRAN 90, is developed to solve hydrodynamics and mass distribution equations, numerically. The use of staggered grids and a control volume method is the main characteristic of the new code. However, the pressure field is corrected at each step by the SIMPLEX algorithm introduced by Patanlar [22]. To enhance numerical stability, the convection terms are approximated by using a hybrid (upwind/central) scheme. In addition, an under-relaxation factor for all dependent variables must be taken to control the strong tendency for divergence. Nevertheless, as the turbulent production term in transport equations for \( k \) and \( \varepsilon \) [1] is non-linear and the velocity gradients at primary steps are so digressive, the eddy viscosity must be limited to less than 2500 \( \nu \) and under-relaxed in each iteration.

To ensure that the non-linear turbulence model is applied correctly in the new code, a fully developed turbulent flow in a channel, as a simple test case, is simulated. The normal stresses obtained by the code using different models are compared with experimental data of Laufer [13] in Figure 1. In contrast with the non-linear model, the linear model has the same estimation of both normal stresses. These results simultaneously validate both the non-linear model and the developed code itself.

In order to discretize Equation 4, it is required that the Reynolds stresses be determined at the main
nodes where \( \bar{p}, \varepsilon, k, \text{ and } C \) are stored. But, at the main nodes near the boundary, the Oldroyd derivative terms could not be approximated via a central difference scheme, which has a second-order error. Hence, the Reynolds stresses are extrapolated there. The validity of this simplification could be explained by noting the general behavior of turbulent flows near the boundary and linear variations of the stresses, there, in fully developed turbulent channel flows (Figure 1). In addition, for the staggered grid [23] used in the calculations, the shear stresses must be known at the cell faces instead of the main nodes where scalar variables are stored. Interpolating could easily solve this problem, but the stress consistency between the two adjacent cells must not be forgotten. This means that four surrounded main nodes are required to interpolate the shear stress at the cell face.

To control the strong divergence, because of the application of the non-linear \( k - \varepsilon \) model, two strategies are taken into account in the solution. First, the calculated flow field, by the linear \( k - \varepsilon \) model, is introduced as an initial condition. Second, the effects of non-linear terms in the anisotropic model are gradually contributed to the solution process by imposing a limitation condition on them. The upper bound of this limitation is defined by comparing the non-linear terms with their corresponding linear terms (i.e., two right hand side terms) in Equation 4. It is worth noting that the modified terms in Equation 4 have more dispersive features than the favorable dissipative one. Thus, in solving the flow field, by using the non-linear \( k - \varepsilon \) and with the initial condition obtained by the standard \( k - \varepsilon \), the averaged-mean-square errors resulting from mass imbalances in control volume will not be drastically reduced. Hence, the calculation process is continued until the difference in length of the separation point of the successive iterations is less than 1%.

In order to calculate the FTC of the tank, Equation 7 is discretized using a fully implicit time discretization method proposed by Roache [24]. Although this method is only first order, an accurate calculation could be achieved by choosing a reasonable time step, which should be determined case by case. Nevertheless, dominant errors of approximating convection terms which have shown themselves as “so called numerical or false diffusion” are still present, which virtually increase the physical diffusion. In fact, as Roache [24] has shown, the solved difference equations will give the solution of the equation below instead of Equation 7, when convection terms are approximated by using the hybrid scheme:

\[
\frac{\partial C}{\partial t} + \frac{\partial uC}{\partial x} + \frac{\partial vC}{\partial y} = \frac{\partial}{\partial x} \left( \Gamma_t + \Gamma_{\text{num}} \right) \frac{\partial C}{\partial x} + \frac{\partial}{\partial y} \left( \Gamma_t \frac{\partial C}{\partial y} \right) + \text{HOT},
\]

where HOT stands for Higher-Order Terms. \( \Gamma_{\text{num}} \) is the numerical diffusivity from the numerical approximation errors. Numerical diffusion is neglected in the \( y \) direction in Equation 8 because the flow in the tank is nearly unidirectional and the cross-stream convection is small.

A practical way to eliminate any numerical diffusion, especially in unsteady problems, is to incorporate more accurate differencing schemes. However, using higher-order schemes has caused stability problems often called wiggles in literature, which limit the application of more accurate schemes. So, as a first approximation, the most popular second-order QUICK is recognized as being suitable for the FTC calculation of the Karlsruher tank. In addition, due to an increase in the bandwidth of the linear equations system by using the standard QUICK scheme of Leonard [25] the QUICK scheme of Hayase et al. [26] is used here.

**FLOW FIELD PREDICTION USING THE MODEL**

Figure 2 shows the dimensions of the Karlsruher model tank, which is selected for flow consideration at Re = 10000 (defined with respect to \( h_0 \), inlet slot high). Its experimental measurements were given by Adams and Rodi [4].

For flow computations of the surface discharge case, \( S_i/h = 0.91 \), a 75 x 35 grid was sufficient to give grid independent solutions. This could be further examined through Figure 3, which shows root mean square errors in different entrance cross sections for u-velocity and eddy viscosity profiles. Errors for any
Hydraulic Efficiency of Primary Settling Tanks

Figure 2. Dimensions of Karlsruhe basin; in surface discharge case $S_I = 10$ cm and in submerged discharge case $S_I = 6.468$ cm.

![Figure 2](image1.png)

Figure 3. $u$-velocity and eddy viscosity error in different cross sections for surface discharge case. (a) Linear; (b) Non-linear $k-\varepsilon$ model. The reattachment length is the same as that reported by Adams and Rodi [4]. This predicted length is only at the lower range of the experiments ($x_r/h = 5.5 - 6.5$). This can be related to the incapability of the linear model. In contrast, in the flow field, which is calculated by non-linear $k-\varepsilon$, as illustrated in Figure 4b, the length of the separation point is increased and reaches $x_r/h = 6.14$. This is now in the experimental range completely and shows clearly the effectiveness of the applied model.

However, the performance of the non-linear model could be seen more obviously if the submerge discharge case, $S_I/h = 0.588$, is considered. Due to the complication of this flow, a grid independent solution is not achieved for both linear and non-linear models with the same grid size (Figure 5). This can also be seen in Figures 6 and 7 that show the calculated streamlines with two different grids. Although using a finer grid with the linear $k-\varepsilon$ has not caused much difference in the flow field (Figures 6a and 7a), the difference in the size of the recirculation bubble is considerable in the case of the non-linear model (Figures 6b and 6b). Therefore, it is inferred that when the non-linear $k-\varepsilon$ is applied for a complicated flow field, the corresponding size of the grid must be about four times greater than the size of a grid that was given a grid independent solution with the linear $k-\varepsilon$ model. This will cause the computation time for the non-linear
model to be dramatically increased. Table 1 shows that when the same grid is used for both models (in surface jet), the non-linear model has a computational time approximately two times greater than the linear model. However, for a submerged jet, a finer grid is required for the non-linear model, so that it has about 30 times greater computational time. This could only be justified in cases where high accuracy is needed e.g. in tank performance calculations (next section).

For the case $S_i/h = 0.588$, two small and large separation zones exist in the flow. Their calculated length, using two models, is compared in Figure 7. Although the modification in the size of the upper small separation zone is not so noticeable, the size of the lower large separation zone is increased from $x_r/h = 4.37$, for linear $k-\varepsilon$, to $x_r/h = 6.0$, for non-linear $k-\varepsilon$, and well approaches the experimental value $(x_r/h)_e = 6.17$. Hence, the non-linear model, without any regard to the flow type and its components...
Table 1. Computational time and separation length error of different grids.

<table>
<thead>
<tr>
<th>Grid Size</th>
<th>Linear $k - \varepsilon$</th>
<th>Reattachment Length Error</th>
<th>CPU Time (s)</th>
</tr>
</thead>
<tbody>
<tr>
<td>$x_r/h$</td>
<td></td>
<td></td>
<td></td>
</tr>
<tr>
<td>$19 \times 9$</td>
<td>5.10</td>
<td>9.0%</td>
<td>8</td>
</tr>
<tr>
<td>$38 \times 18$</td>
<td>5.30</td>
<td>3.7%</td>
<td>38</td>
</tr>
<tr>
<td>$75 \times 35$</td>
<td>5.49</td>
<td>2.0%</td>
<td>646</td>
</tr>
<tr>
<td>$150 \times 70$ (Reference)</td>
<td>5.6</td>
<td>0.0%</td>
<td>9267</td>
</tr>
</tbody>
</table>

Non-Linear $k - \varepsilon$

<table>
<thead>
<tr>
<th>Grid Size</th>
<th>$x_r/h$</th>
<th>Reattachment Length Error</th>
<th>CPU Time (s)</th>
</tr>
</thead>
<tbody>
<tr>
<td>$19 \times 9$</td>
<td>5.65</td>
<td>8.9%</td>
<td>17</td>
</tr>
<tr>
<td>$38 \times 18$</td>
<td>6.04</td>
<td>2.6%</td>
<td>77</td>
</tr>
<tr>
<td>$75 \times 35$</td>
<td>6.14</td>
<td>1.0%</td>
<td>892</td>
</tr>
<tr>
<td>$150 \times 70$ (Reference)</td>
<td>6.20</td>
<td>0.0%</td>
<td>18780</td>
</tr>
</tbody>
</table>

Exp. Adams et al. [4]

Submerged Discharge Case

<table>
<thead>
<tr>
<th>Grid Size</th>
<th>Linear $k - \varepsilon$</th>
<th>Reattachment Length Error</th>
<th>CPU Time (s)</th>
</tr>
</thead>
<tbody>
<tr>
<td>$38 \times 26$</td>
<td>3.2</td>
<td>27.0%</td>
<td>74</td>
</tr>
<tr>
<td>$75 \times 52$</td>
<td>4.32</td>
<td>1.3%</td>
<td>878</td>
</tr>
<tr>
<td>$150 \times 100$</td>
<td>4.37</td>
<td>0.2%</td>
<td>13210</td>
</tr>
<tr>
<td>$300 \times 200$ (Reference)</td>
<td>4.38</td>
<td>0.0%</td>
<td>88122</td>
</tr>
</tbody>
</table>

Non-Linear $k - \varepsilon$

<table>
<thead>
<tr>
<th>Grid Size</th>
<th>$x_r/h$</th>
<th>Reattachment Length Error</th>
<th>CPU Time (s)</th>
</tr>
</thead>
<tbody>
<tr>
<td>$38 \times 26$</td>
<td>3.83</td>
<td>36.7%</td>
<td>135</td>
</tr>
<tr>
<td>$75 \times 52$</td>
<td>5.18</td>
<td>14.4%</td>
<td>1700</td>
</tr>
<tr>
<td>$150 \times 100$</td>
<td>6.00</td>
<td>0.8%</td>
<td>25000</td>
</tr>
<tr>
<td>$300 \times 200$ (Reference)</td>
<td>6.05</td>
<td>0.0%</td>
<td>190000</td>
</tr>
</tbody>
</table>

Exp. Adams et al. [4]

$u = \frac{\partial \psi}{\partial x}$ as:

$$u \frac{\partial}{\partial x} (\nabla^2 \psi) + v \frac{\partial}{\partial y} (\nabla^2 \psi) = (\nu + \nu_1) \nabla^4 \psi$$

$$- \frac{\partial^2}{\partial x \partial y} (\tau_{22} - \tau_{11}) - \frac{\partial^2 \tau_{12}}{\partial x^2} + \frac{\partial^2 \tau_{12}}{\partial y^2} = 0$$

This equation shows that in cases where high velocity gradients are present, the linear $k - \varepsilon$ model is unable to predict the normal Reynolds stress difference, $\tau_{22} - \tau_{11}$. This term contributes directly to calculating the streamlines and their curvatures. It can be shown that the linear model promises the sum of the normal Reynolds stresses as $(\tau_{11} + \tau_{22} = 2\tau_{12})$. For any 2-D flows, that surely will not satisfy all cases. In other words, the linear model has some problems in calculating the normal stresses.

Figure 8a compares the measured and calculated dimensionless streamwise velocity, $u/u_{in}$, at several critical sections in the recirculation zone for the submerged discharge case (the case in which the inflow is from the middle height of the tank). The superiority of the non-linear model, especially in the regions near the separation point, $x/h = 5.58, 5.88$ and $6.17$, is clearly apparent. Furthermore, Figure 8b gives additional information about the status of turbulence complexity, acts quite well in predicting the size of large recirculation zones, which are more interesting. The model only fails in simulating small recirculation zones where the curvature of streamlines is extraordinarily strong. These small separation bubbles are not more important, as they have negligible influence on the flow characteristics.

The reason why the standard turbulence model is unsuccessful in predicting separated flows will be clearer when momentum equations are written in terms of the mean flow stream function, $\psi$, (where $u = \frac{\partial \psi}{\partial y}$).

Figure 8. Dimensionless velocity (a) and turbulence kinetic energy (b) profiles at several critical sections in recirculation zone of Karlsruhe tank in submerged discharge situation.
in the tank. In this figure, the calculated dimensionless
 turbulence kinetic energy (divided by the square mean
 inlet velocity, \( u_{i0}^2 \)) using both standard and non-linear
 \( k - \varepsilon \) models, is compared with the experimental data
 of Adams and Rodi [4]. As expected, the highest
 turbulence levels are found in shear layers bordering
 the separation zones (i.e. \( x/h < 6.2 \)). Beyond
 reattachment, the turbulence level drops quickly to
 about a constant small value due to the absence of any
 significant velocity gradients. This simply explains why
 the standard \( k - \varepsilon \) model has very poor predictions
 of turbulence kinetic energy levels within the separation
 zone, while having a good performance out of this
 region. As shown earlier, the section, \( x/h = 5.58 \),
 for a submerged discharge case, is entirely out of
 the separation region when the linear \( k - \varepsilon \) model
 is incorporated. In contrast, the separation zone has
 extended beyond this section for the non-linear \( k - \varepsilon \)
 model and for measurements. Therefore, nearly good
 agreement is observed between the non-linear \( k - \varepsilon \)
 model and experimental data for this critical section.
 However, separation has ended at this section and,
 consequently, the turbulence level has dropped for the
 linear \( k - \varepsilon \) model.

CALCULATING THE FTC

The hydrodynamic performance of a tank is obtained
 by injecting fluorescent dye with the same density
 as the internal water at a definite time in the inlet
 and measuring the outlet dye concentration several
 times. A curve that shows the dye concentration
 variation in the outlet with time is called a Flow-
 Through Curve (FTC). It is more convenient to present
 the FTC with dimensionless variables. Hence, time
 is non-dimensionalized by the theoretical detention
 time, \( T_{th} = \frac{h^2}{q} \), where \( q \) is the inlet flow rate, and
 concentration by \( C_0 = \frac{C(t)}{C_{in}} \), where \( C_{in} \) is the total mass
 of the dye to enter the tank.

To calculate the FTC of the Karlsruhe tank, the
 concentration transport Equation 7 must be solved by
 introducing the computed flow field in the previous
 section. In addition, for simplicity, the inlet boundary
 condition is taken as \( C = 1 \) during the injection time
 and \( C = 0 \) for the next times.

Figure 9a compares the calculated FTC for a
 surface discharge situation, with both hybrid and
 QUICK schemes, using two different flow fields ob-
 tained by linear and non-linear models, with Adams
 and Rodi’s [4] measurements. In this case, the peak
 concentration predicted with the hybrid scheme
 and non-linear \( k - \varepsilon \) model is almost the same as that
 measured, while the peak concentration predicted by
 the QUICK scheme and the non-linear \( k - \varepsilon \) model
 is about 35\% higher. However, this value is more
 (about 60\%) when a linear \( k - \varepsilon \) model is used. The
 discrepancy between the numerically accurate QUICK
 calculation and the measurements may be due to three
 reasons. The first possibility is that the incorporated
 turbulence model in the hydrodynamic part cannot
 produce sufficient mixing, i.e. eddy diffusivity as it is
 produced in the experiment. According to Figure 9b,
 this is the case, but it does not solely justify the
 poor prediction of the QUICK scheme. The level
 of the turbulence fluctuations predicted by the linear
 \( k - \varepsilon \) model is generally lower than the measurements
 and the non-linear turbulence model. Therefore, the
 FTC calculation for the flow field obtained by the
 non-linear \( k - \varepsilon \) model can reduce peak concentration
 to about 25\%. This is high, but not as much as
 expected. The second possibility for the difference
 in peak concentration may be due to the inconsis-
 tency of the QUICK scheme with the physics of the
 problem. The QUICK scheme is often used in steady
 state calculations (also it has been used for transient
 computations by some researchers), while many other
 schemes have been proposed originally for transient
 calculations. To discover the effect of unsteadiness
 in the calculations, we have repeated the FTC calculation
 with the third order accurate QUICKEST scheme
 developed originally by Leonard [25]. To avoid the oc-
 currence of non-physical numerical oscillations, a mod-
 ified ULTIMATE algorithm has also been used [27].
 The result (Figure 9b) is observed as a lowering in
 peak concentration by an amount of 8\% relative to
 the QUICK scheme. Hence, another possibility may
 also exist. Based on the statements of Adams and
 Rodi [4], three-dimensional motions were observed
 in the experimental tank, in spite of their attempt to
 keep the motion two-dimensional through the large aspect
 ratio of the tank. Definitely, additional mixing due to
 these three-dimensional motions is not reproducible by
 using the two-dimensional model.

Figure 9c, for a submerged discharge case, shows
 that the peak concentration is highest for the linear
 \( k - \varepsilon \) with the QUICK scheme and lowest for the
 non-linear model with the hybrid scheme. This in-
 situation, when the size of the recirculation zone is
 enlarged, the peak concentration decreases, as shown
 previously. But, the passing time of peak concentration
 decreases.

Figure 9d shows a comparison between the FTC
 of the two different cases, which were calculated using
 precise flow fields and a QUICK scheme without nu-
 merical diffusion. In the submerged discharge case, the
 FTC is changed in such a way that it approaches the
 ideal step FTC that could be considered as a sign of
 higher hydraulic efficiency.

Through the FTC, some important parameters,
 such as the level of short-circuiting, the mixing level
 influenced by both diffusion and/or recirculation zones,
and overall efficiency, could be calculated. The time it takes for the first dye to appear at the outlet is defined as the short-circuiting level and is shown by \( t_0 \) and \( t_{10} \) indices (\( t_n \) is the time which takes until \( n \) percent of the total injected dye into the tank is passed through the outlet). The value of these indices depends on both the flow field and the mixing level. When the values are low, one may conclude that short-circuiting exists and it shows a false designing of the tank. The mixing level in the tank, defined by \( t_{20} - t_{30} \), \( t_{60} - t_{10} \) and \( t_{60}/t_{10} \), is increased by increasing the diffusion coefficient and the size of the recirculation bubble. Nevertheless, the performance of the tank is increased by the former and decreased by the latter. \( t_{60} \) and \( t_{\text{max}} \), the passing time of peak concentration, are indices to express the overall efficiency of the tank.

All the computed FTC characteristic indices of the Karlsruhe tank are summarized in Table 2. The short-circuiting indices are decreased or, in other words, the level of short-circuiting is increased by increasing the separation bubble (compare columns 1 with 2 or 4 with 5) and the virtual diffusion coefficient created by additional numerical diffusion (compare columns 2 with 3, or 5 with 6). In addition, Table 2 shows that the tank’s mixing level is increased when the above-mentioned factors increase. This high mixing disturbs the still circumstances, which are required for efficient settling. Altogether, it is inferred from Table 2 that increasing the recirculation size causes a high level of short-circuiting and mixing. The result of these complicated events can be seen on the reduction of \( t_{60} \) and \( t_{\text{max}} \) indices that indicate tank performance. Conversely, the efficiency of the tank is not more sensitive to the diffusion coefficient; however, the mixing level in the tank is increased by its increasing (see columns 2, 3 or 5, 6 of Table 2).
Table 2. FTC indices of Karlsruhe tank.

<table>
<thead>
<tr>
<th>FTC Index</th>
<th>$S_h/h = 0.588$</th>
<th>$S_h/h = 0.91$</th>
</tr>
</thead>
<tbody>
<tr>
<td></td>
<td>Standard $k - \varepsilon$ Quick</td>
<td>Non-Linear $k - \varepsilon$ Quick</td>
</tr>
<tr>
<td>Short Circuiting</td>
<td></td>
<td></td>
</tr>
<tr>
<td>$t^0$</td>
<td>0.67</td>
<td>0.653</td>
</tr>
<tr>
<td>$t_{90}$</td>
<td>0.965</td>
<td>0.843</td>
</tr>
<tr>
<td>Mixing</td>
<td></td>
<td></td>
</tr>
<tr>
<td>$t_{15} - t_{25}$</td>
<td>0.225</td>
<td>0.258</td>
</tr>
<tr>
<td>$t_{90} - t_{10}$</td>
<td>0.513</td>
<td>0.577</td>
</tr>
<tr>
<td>$t_{90}/t_{10}$</td>
<td>1.933</td>
<td>1.684</td>
</tr>
<tr>
<td>Efficiency</td>
<td></td>
<td></td>
</tr>
<tr>
<td>$t_{\text{max}}$</td>
<td>0.965</td>
<td>0.942</td>
</tr>
<tr>
<td>$t_{90}$</td>
<td>1.023</td>
<td>1.01</td>
</tr>
</tbody>
</table>

CONCLUSION

This paper has presented the main reasons for deficiency in the standard $k - \varepsilon$ turbulence model in prediction of the separation point length of the flow in the Karlsruhe tank. The non-linear $k - \varepsilon$ model was used to overcome the weakness of the linear model and to calculate the size of the recirculation zone exactly. Comparing the computed streamwise velocity profiles with measurement data in several critical sections shows that the non-linear model is more effective. In addition, incorporating the velocity fields, obtained by the two different turbulence models, shows that a precise flow field is the main requirement for calculating an accurate FTC. If the size of the separation bubbles in the Karlsruhe is predicted slightly larger, a reduction will appear in the tank’s overall efficiency. However, this reduction is not very notable because the size of the recirculation zone is small, with respect to the tank’s volume, and strong 3-D effects were present in the tank.

To consider the complicated effects of the recirculation zones, the non-linear $k - \varepsilon$ model was used to calculate the FTC. This is reasonable because the non-linear model has no increased computation costs and can be incorporated simply into codes with the standard $k - \varepsilon$ model.

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NOMENCLATURE

- $C$: mean dye concentration
- $C_0$: reference concentration
- $C_D$: coefficient of non-linear terms in Reynolds stresses formula
- $C_{z1}, C_{z2}$: experimental constants in $\varepsilon$ transport equation
- $C_{\mu}$: dimensionless constant in Reynolds stresses formula
- $E(X)$: root mean squares error of variable $X$ in a selected cross section
- $h$: depth of basin
- $h_i$: width of inlet slot
- $k$: kinetic energy of turbulence
- $L$: length of basin
- $m_{\text{in}}$: total injected mass of dye
- $n$: normal distance from the solid wall
- $N$: number of data in each cross section to calculate $E(X)$ ($\approx 20$)
- $P$: turbulence production term
- $\bar{p}$: modified mean pressure
- $q$: inlet flow rate
- $Re$: Reynolds number defined with respect to inlet slot high
- $S_h$: location of inlet slot above tank bottom
- $S_{ij}$: mean rate of strain tensor
- $S^p_{ij}$: frame-indifferent Oldroyd derivative
- $T_{th}$: theoretical detention time
- $t$: time
- $t_{\text{max}}$: passing time of the peak concentration
Hydraulic Efficiency of Primary Settling Tanks

u, v mean velocity components in the x and y directions
\( u_{in} \) inlet velocity
x, y streamwise and vertical direction
\( x_r \) separation point length
\( y_p \) normal distance to the wall from wall adjacent grid point
\( \delta_{ij} \) Kronecker delta
\( \Gamma_t \) isotropic eddy diffusivity coefficient
\( \Gamma_{num} \) numerical diffusivity
\( \varepsilon \) turbulent dissipation rate
\( \nu \) kinematic viscosity
\( \nu_t \) isotropic eddy viscosity
\( \sigma_{\varepsilon} \) turbulent Schmidt number for dye
\( \sigma_{\varepsilon_k} \) Schmidt number for k and \( \varepsilon \)
\( \tau_{xx}, \tau_{yy} \) normal components of Reynolds stresses
\( \tau_{xy} \) shear component of Reynolds stress
\( \psi \) mean flow stream function normalized by flow rate (q)

Subscripts
1, 2 x and y directions, respectively
n percent of total injected dye which has passed through outlet
e experimental data
ref selected reference grid

REFERENCES


BIOGRAPHIES

Bahar Firoozabadi is associate professor in the school of mechanical engineering at Sharif University of Technology, Tehran. Her research interests are fluid mechanics in density currents, presently focusing on bio fluid mechanics, and porous media. She received her PhD in mechanical engineering also at Sharif University. She teaches fluid mechanics and gas dynamics for undergraduates, and viscous flow, advanced fluid mechanics, continuum mechanics and biofluid mechanics for graduate students.

Mohammad Ali Ashjari was born in 1977 at Tabriz, centre of East Azarbayjan province, Iran. His academic program has been started since 1996 when he has entered Tabriz University to study Mechanics of Fluids. He had continued his education in Sharif University of Technology (SUT) in M.Sc. degree. He has received PhD degree in Thermo-Fluid from SUT in 2008. Dr. Ashjari’s interest research field is developing and introducing novel numerical methods for fluid flow simulation under complicated conditions including turbulence and multi-phase porous media. He is now faculty member of Jolfa’s Azad University.
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